

Subject: Circular Concrete Shaft Shear Capacity Sponsor: TxDOT

1.) Given the following Design Data:

 $f_c := 3600psi$ Compressive Strength

of Concrete (psi)

 $f_y := 60000psi$ Yield Strength

of Rebar (psi)

Information From TxDOT Standards for Different Columns Used:

Diameter of Column from TxDOT Standards for Different Bents & Roadway Widths

$$Dia_{col} := \begin{pmatrix} 24\\30\\36\\42 \end{pmatrix} in$$

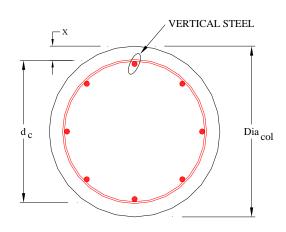
Diameter of Spiral Stirrup (in.) for Columns Size above, (TYP.)

Stirrup_{size} :=
$$\begin{pmatrix} 0.375 \\ 0.375 \\ 0.375 \\ 0.625 \end{pmatrix}$$
 in

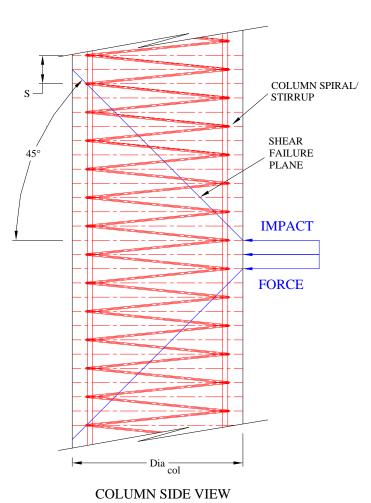
Size of Vertical Steel (All Cases)

$$Vertical_{size} := \frac{9}{8}in$$

$$\begin{aligned} \textbf{d}_{\textbf{C}} &\coloneqq \text{Dia}_{\textbf{COl}} - 2 \cdot \textbf{X} & \quad \text{Diameter of Spiral} \\ &\quad \text{Steel} \end{aligned}$$



COLUMN SECTION





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$$d_{c} = \begin{pmatrix} 18 \\ 24 \\ 30 \\ 36 \end{pmatrix} \cdot \text{in}$$
 Diameter of Spiral Steel (in.)

 $s_{spa} := 6in$ Pitch in Spiral Stirrup

 $\gamma_{con} := 150 pcf$ Unit weight of concrete (lbf/ft³)

2.) Calculate V_c ~ Strength Attributable to Concrete:

Members with circular cross-sections, such as columns, are designed to resist crash loading (shear force). When circular ties or spirals are used as web reinforcement, the calculation of V_c is based on ACI Equation 11-4, ACI-318R-05

with:

$$Dia_{col} = \begin{pmatrix} 24 \\ 30 \\ 36 \\ 42 \end{pmatrix} \cdot in \qquad Diameter of Column (in.)$$

$$A_{g} := \frac{\pi \left(Dia_{col}\right)^{2}}{4} \qquad A_{g} = \begin{pmatrix} 452.389 \\ 706.858 \\ 1017.876 \\ 1385.442 \end{pmatrix} in^{2}$$

$$N_u := 50 \text{ft} \cdot 15 \text{ft} \cdot 12 \text{in} \cdot \gamma_{con}$$
 $N_u = 112.5 \text{ kips}$

Gross Area of column (in²)

Approximate axial force normal to the cross-section occurring simultaneous with the crash impact force on the column. Assume a certain area dead loading from the slab at a certain size and thickness.



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Compressive strength of column concrete (psi) $f_{c} = 3600 \cdot psi$

Approximate distance from extreme compression fiber to the centroid $d := 0.8 \cdot Dia_{col}$ of the longitudinal tension reinforcement, but need not be taken greater than 0.8Dia_{col} see ACI 318-05, Section 11.3.3

$$V_{c} := \boxed{2 \left(1 + \frac{N_{u}}{2000 \cdot \text{psi} \cdot A_{g}}\right) \cdot \sqrt{\frac{f_{c}}{\text{psi}}} \cdot \text{psi} \cdot \text{Dia}_{col} \cdot d \cdot 2} \qquad \begin{array}{c} \text{See ACI 318-05, Section} \\ 11.3.1.2, \text{ Equation 11-4} \\ \dots \text{ times 2 } \dots \text{ for 2 shear planes} \end{array}$$

$$Dia_{col} = \begin{pmatrix} 24 \\ 30 \\ 36 \\ 42 \end{pmatrix} in \qquad V_{c} = \begin{pmatrix} 124.343 \\ 186.551 \\ 262.583 \\ 352.439 \end{pmatrix} \cdot kips \qquad Nominal shear strength of concrete alone for corresponding column Dia.$$

3.) <u>Calculate V_s ~ Strength Attributable to Shear Reinforcement:</u>

As per ACI-318R-05, Section 11.5.7.3 - Where circular ties, hoops, or spirals are used as shear reinforcement, Vs shall be computed using Eq. 11-15 where "d" is defined in 11.3.3 for circular members, Av shall be taken as two times the area of the bar in the circular tie, hoop, or spiral at a spacing "s", "s" is measured in a direction page to the longitudinal reinforcement, and "f_{vt}" is the specified yield strength of the circular tie, hoop, or dpiral reinforcement.

Calculate the Area of the Spiral Steel (in²)

$$A_{v} := \frac{\pi \cdot \text{Stirrup}_{\text{Size}}^{2}}{4} \cdot 2$$

$$A_{v} = \begin{pmatrix} 0.221 \\ 0.221 \\ 0.221 \\ 0.614 \end{pmatrix} \cdot \text{in}^{2} \quad \text{Area of Stirrup Steel (in}^{2}) \dots \text{ times } 2 \text{ as per ACI}$$

$$318R-05, \text{ Section } 11.5.7.3$$

yield strength of spiral reinforcement (ksi)



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 $s_{spa} = 6 in$ Spiral spacing / pitch (inches)

$$V_{s} := \underbrace{\left(\frac{A_{v} \cdot f_{yt} \cdot Dia_{col}}{s_{spa}} \cdot 2\right)}_{spa}$$
 See ACI-318R-05, Section 11.5.7.2 use times 2 ... for 2 failure planes

4.) Calculate the Nominal Shear Capacity of Column for two Failure Plane Mechanism considering the strength of the concrete and the spiral reinforcing steel:

$$V_n := V_c + V_s$$

Concrete Shear Strength

Spiral Reinforcment Strength

$$V_{c} = \begin{pmatrix} 124.343 \\ 186.551 \\ 262.583 \\ 352.439 \end{pmatrix} \cdot \text{kips} \qquad V_{s} = \begin{pmatrix} 106.029 \\ 132.536 \\ 159.043 \\ 515.418 \end{pmatrix} \cdot \text{kips}$$

$$V_{S} = \begin{pmatrix} 106.029 \\ 132.536 \\ 159.043 \\ 515.418 \end{pmatrix} \cdot \text{kips}$$

Column Dia.

$$Dia_{col} = \begin{pmatrix} 24\\30\\36\\42 \end{pmatrix} \cdot in$$

$$Dia_{col} = \begin{pmatrix} 24 \\ 30 \\ 36 \\ 42 \end{pmatrix} \cdot in \qquad V_{n} = \begin{pmatrix} 230.372 \\ 319.087 \\ 421.626 \\ 867.857 \end{pmatrix} \cdot kips$$

Nominal Shear Capacity of Column (kips)

Alpha Calculations